# FINDS ELEMENTARY RULES OF ENGINEERING SLIGHTED

(Continued From First Page.)

leaking of melted snow onto the

sulted. In figure 1 the double lines indicate the six roof trusses, while the single lines indicate the steel I-beams or girders connecting with the trusses. Four of the trusses connected at one end of each to the main truss, No. 11. One truss, No. 16, extended from the east wall to column No. 5. The main truss, No. 11, was at right angles to the other trusses, and its north end rested upon the curved wall on the Columbia road side of the building. Its south end was supported on the upper end of a column (No. 2), at the level of the ceiling. This column, unlike column No. 3, did not extend to the roof, but stopped at the level of the suspended ceiling, which was about six or seven feet below the

## Supported by Vertical Strut.

The three roof girders meeting at the point over column No. 2 were supported by a vertical strut, consisting of two small steel angles, which strut stood upon the bottom chord of truss No. 11, directly over the top of column No. 2. It should be observed that there is no steel mem be restending west from the end of Migures will develop that their ber extending west from the end of Migures will not be in turn agreement this strut. This strut was supported with recorded sires at the fallure was due to the pulling apart of any steel section and the writer would not expect such to be the course. this strut. This strut was supported "cobhouse" fashion, not being rigidly connected at its base and not connected directly to the column below, contrary to accepted practice. See

figure 8.

Truss No. 12 is the one which appears in the various photographs of the interior taken since the collapse. It is altogether, but has a bad break in its upper or compression flange.
This break is evidently due to tension developed by a tremendous side pull which probably came with the general collapse, rather than being the cause

collapse, rather than being the cause of the collapse.

Figure 2 is a sketch to larger scale, of the top of column No. 3 and shows the relative positions of the four roof members connecting at this column. Perhaps it is well here to call attention to the fact that all of these connections were made with loose fitting bolts instead of rivets It may be permissible to make steel connections with bolts in a framework of short spans, but good practice dictates the use of field driven rivets in the connections for work of this character. Where bolts are used the number should always be greater than would suffice for rivets, which is not the case here.

should always be greater than would suffice for rivets, which is not the case here.

Figure 3 is an elevation or side view of a part of the connection shown in plan in figure 2. This shows that the twelve-inch I beam "B-41" was connected in a most flimsy fashion to its support, which was a short section channel twelve inches high and half an inch thick, standing on angle brackets, connected to the column.

The upper flange of this channel was not fastened to the column in any way, so that a very slight movement of beam "B-41" would certainly cause the web of the channel to fold over, bending near its bottom flange, and this is just what did shappen to this channel support, as shown by dotted lines on figure 3.

Figure 4 is a sketch of the connecting beams and the angle strut above column No. 2. The west end of beam "B-41" was connected by a bent plate boiled to the 10-inch channel.

Figure 5 shows a side view of this connection to the west end of B-41 to

Figure 5 shows a side view of this connection to the west end of B-41 to the 10-inch channel. This connection was made by six bolts through the bent plate and the web of the channel, and this connection is still included in the weekage. tact in the wreckage.

### Only Three Bolts Carry Load. Figure 6 shows the connection of

this same 10-inch channel to the vertical 3-inch by 4-inch angle by would instantly collapse, with the remeans of three bolts. Notwithstanding the fact that the load from B-41
was delivered to this channel by six
bolts and that the channel was also
called upon to carry in addition a
with the result that it would be car-

Columns No. 2 and 3 were each spliced above the balcony level it similar columns which extended from the foundations up and through the balcony framing. These columns were more slender for their lengths than good practice warrants. The connections between the upper and lower columns was entirely inade-one to the top of columns was entirely inade-one to the top of columns was connected to forces tending to move their top at these columns and when subjected to forces tending to move their top at these columns and when subjected to forces tending to move their top laterally they simply broke away from the ineffective splice plates, just above the balcony.

Figure 8: shows the top of column No. 2. Sketch (a) of figure 8: shows the top of olumn No. 2. Sketch (a) of figure 8: shows the top of column No. 2. Sketch (a) of figure 8: shows the top of column No. 2. Sketch (a) of figure 8: shows the top of column No. 2. Sketch (b) of figure 10: shows the stop of the column and temperature of t



T. L. CONDRON.

pect such to be the case.

### Glaring Faults Found.

Some glaring faults in the sections of the trusses are found. The splicing of both the tension and compreswas inadequate and improperly done, so that the stresses at the splices were nearly double the allowable safe stresses. However, failure did safe stresses. However, failure did not result from this cause, although it was tempting Providence to try such a reckless experiment over the heads of unsuspecting audiences.

Evidently no thought was given the fact that truss No. 11 holted to lug angles on the top of column No. 2, caused large bending stresses in the column No. 2 due to the deflection of the truss itself. This tall, slender column with inadequate connection at its base was unable to resist bending stresses and simply tended to tip on its base and in fact at the collapse did so tip.

It is difficult to determine what the loads and stresses were in the balcony framing. An addition was made to the balcony last summer, the details of which the writer has not seen. A glance at figure 1 will show that any disturbance of the equilibrium of the framing at either of the supports over the two columns. Nos. 2 or 3, would be fatal.

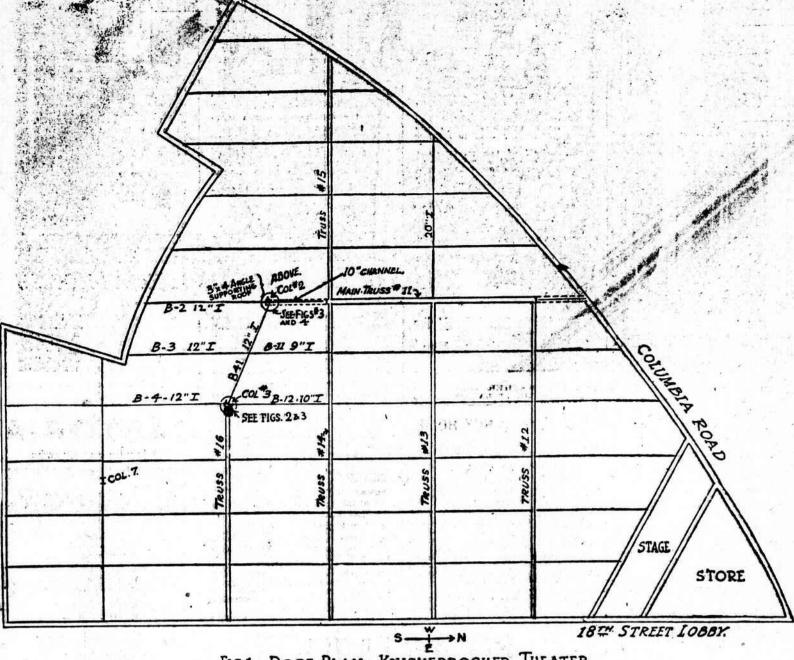
It is evident that beam B-41 is the most seriously overloaded and most poorly supported member of the foof framing. This beam is shown to be of the same size as other beams in the roof, which had much smaller areas to support. Beam B-41 is bent at present, which, while not surprising, is confirmatory of the view that it was bent out of line by severe overloading. Here is a case where the snow load was a larger factor than in the case of the dead load stresses. not result from this cause, although

This beam being so seriously over-loaded would tend to buckle sidewise, and if it started to buckle it would bring a pull upon the top flange of the vertical channel on which its east end rested, and that channel

botts and that the channel was also called upon to carry in addition a considerable area of the roof, only three bolts were provided to transmit ail of this load to the supporting angle. During the collapse this channel was torn away from the angle support and one bolt head was pulled clear through the thin steel web of the channel.

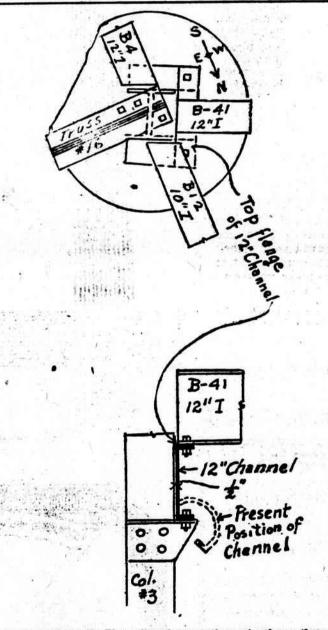
Referring back to figure 1 it will be noted that in case any force tended to disturb the equilibrium of the point of support over column No. 2. and cause a movement of B-41 would tend to upset the light angle strut, which rested so insecurely on top of the rested to the relative positions and the unbraced condition above the balcony of column No. 2 and also of the angle strut above it. When first creeded there was no member in the plane of the ceiling between columns No. 2 and also of the angle strut above it. When first creeded there was no member in the plane of the ceiling between columns No. 2 and also of the angle strut above it. When first creeded there was no trigidly connected by two bolts to the flange of the truss through a flat plate.

Columns No. 2 and 3 were each spliced above the balcony level to similar columns which extended from the foundations up and through the balcony framing. These columns reason of the rested to the foundations up and through the balcony framing. These columns from this it is evident that even the balcony framing. These columns the balcont that even the foundations up and through the balcony framing. These columns the balcont th

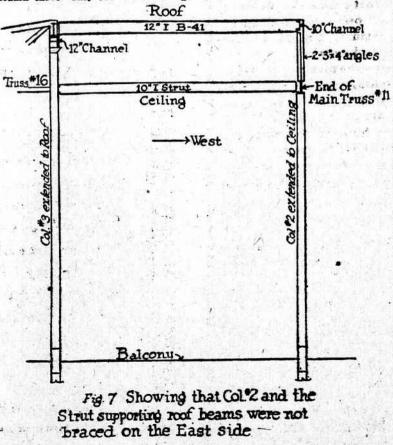


MG.1 ROOF PLAN- KNICKERBOCKER THEATER. Single Lines- Beas or Channel Double Lines- Trusses

7.



Upper: Figure 2—Plan of roof connections at column three.
Lower: Figure 3—Side view of connection of beam B41—to channel at
mn three—only the bottom flange of channel connected to the column.



Polytechnic Institute, where he was a classmate of George R. Putnam, now United States commissioner of lighthouses.

During the past twenty-one years he has been engaged in practice in Chicago as a consulting and designing structural engineer. Among his clients for bridges and similar structures have been a number of railroads, including the Chicago and Eastern Illinois, Illinois Central, M. K. and T., and Mobile and Ohio. A large portion of his work has been in connection with important buildings throughout the country. The Condron to design and direct the repairs and restoration of the large group of buildings gutted by fire at the west Orange plant of the Edison Company.

Will Construct Great Violent.

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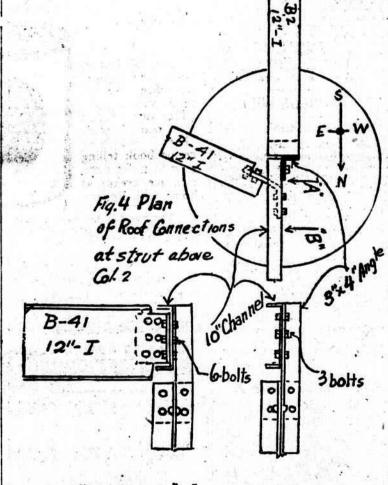
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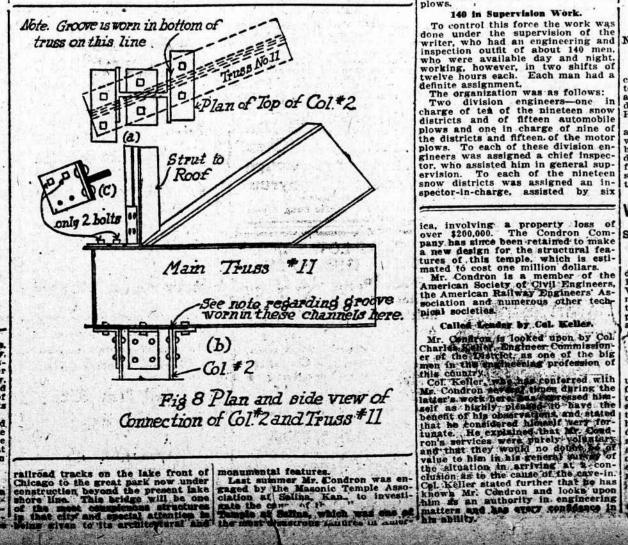
Will Construct Great Violent.

The Condron to the Edison Company.



B-4/a 12"I to the lo"channel. This

Fig. 5 Section at B Fig. 6 Section at A of Fig. 7 Section at A of Fig. 6 Section at A of Fig. 7 Section at A of Fi to angle strut over Col. 2. The top bott cornection is still head in this connection intact as it has was pulled through the thin web of channel in the collapse. There were 3 botts



# DRAWINGS BY THEODORE L. CONDRON EXPLAINING KNICKERBOCKER COLLAPSE. SNOW FOUGHT WHILE FALLING UNDER PHILADELPHIA'S SYSTEM

Fleet of Speedy Motor Plows, 4,390 Men, 1,110 Teams and City's Sewers Keep Streets Cleared.

Evening Star to collect data from various cities concerning their

Divided Into Districts.

The central business portion of the city was divided into nineteen snow districts. comprising about thirty miles of streets. Detailed plans of these districts were prepared showing the sewer manholes which might be used for dumping the snow. Where it was found that there was insufficient flow of water in the sewers, the manholes were fitted with water connection to increase the flow. Bids were received by the city from teaming contractors for the removal of snow within the nineteen snow districts. Not more than two districts were awarded to any one contractor. The basis of payment was per cubic yard of snow disposed of in a manhole or other dumping place indicated on the plans. The contractors were subject to call, and were required under penalty to have a certain number of men and teams at work within their respective snow districts within one hour after notification to begin work. The total force required for the nineteen districts was 2,000 men and 800 teams and trucks.

In addition, the regular street cleaning force was also organized for snow work. This force included 1,200 men and 200 teams and trucks. They were required to clean the crossings in the central part of the city and to remove snow in the outlying sections. The municipal repair and sewer gangs consisting of 1,110 men and 110 teams and trucks, were an extra force available for snow work in severe storms, thereby making a grand total of the snow force of 4,330 men and 1,110 men and 110 teams and trucks, thirty heavy automobile plow trucks and eight trolley plows.

WILL CONSULT HOVER.

To control this force the work was done under the supervision of the writer, who had an engineering and inspection outfit of about 140 men, who were available day and night, working, however, in two shifts of twelve hours each. Each man had a definite assignment.

The organization was as follows:
Two division engineers—one in charge of tea of the wind of the minute of the charge of tea of the wind of the minute of the working the work was and charge of the working th

Two division engineers—one in charge of ten of the nineteen snow districts and of fifteen automobile plows and one in charge of nine of the districts and fifteen of the motor plows. To each of these division en-gineers was assigned a chief inspec-tor, who assisted him in general sup-ervision. To each of the nineteen snow districts was assigned an in-spector-in-charge, assisted by six

ica, involving a property loss of over \$200,000. The Condron Company has since been retained to make a new design for the structural features of this temple, which is estimated to cost one million dollars.

Mr. Condron is a member of the American Society of Civil Engineers, the American Railway Engineers' Association and numerous other technical societies. pical societies.

Called Lender by Cal. Keller.

BY COL. JAMES B. McCORD.

I. S. Bureau of Public Roads, Formerly Engineer of Highways, Philadelphia, Pa.

The writer has read with a great
leal of interest the efforts of The
Evening Star to collect data from
various cities concerning their
pethods and openization for snow

subordinate inspectors. Each inspector in charge of a district was provided, of course, with a set of plans and specifications and was required to keep in close touch with his contractor by telephone, to be familiar with the location of the plow truck garages and the addresses of the crews. The police precincts also coportated in making up and mobilizing contractors and city laborers for

The writer has read with a great deal of interest the efforts of the read with a great deal of interest the efforts of the read writer was to collect data from methods and organization for snow removal. It is thought that it might he of interest to your readers to operation in the nearby city of Philadelphia. The writer was in charge of the dapkins for the years here in 1912 and 1917 the writer was in charge of the dapkins for the years and the property of t

Fifty secretaries and presidents of chambers of commerce of thirty-five towns and cities in New England have arrived in Washington for a two-day conference with Secretary Hoover.

The conference is to be held today and tomerrow. Secretary Hoover will describe the work being done by the bureau of foreign and domestic commerce in promoting foreign trade. The present economic situation with respect to foreign trade also will be discussed.

## WILL DISCUSS FORD OFFER

Secretary Weeks to Appear Before House Military Committee.

Secretary Weeks announced today that he would appear before the
House military affairs committee
Wednesday at 10:50 o'clock to supplement orally his report on the contract proposal of Henry Ford for
purchase and operation of the nigrate
and power projects at Muscle Shorls,
Ala., recently sent to the House.

## GEN. HARBORD ENDS TRIP.

Maj Gen. James G. Harbord, deputy chief of staff, who accompanied Col. Wainwright, assistant secretary of war, on an extensive tour of inspection of military posts and stations in the south and west, returned here this morning and resumed his duties; at the War Department. Assistant Secretary Wainwright, who extended his trib to St. Paul and other points in the northwest, is not expected to return to this caty before next Monday.